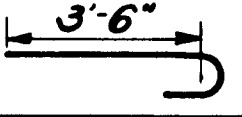
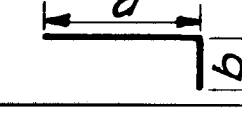
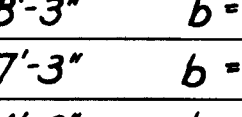
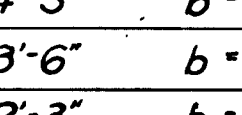
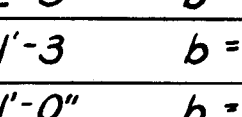
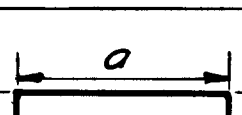
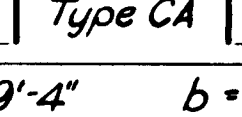
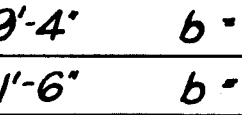
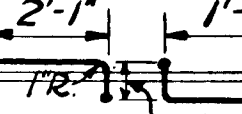
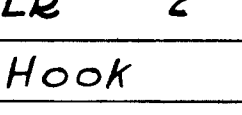
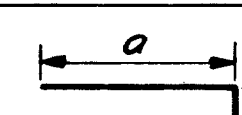
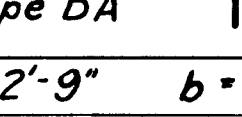
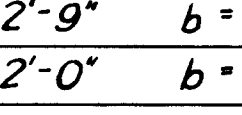
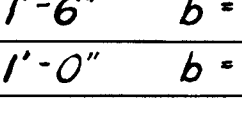
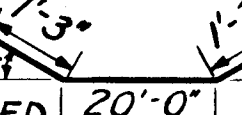
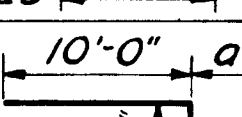
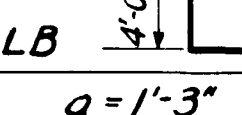
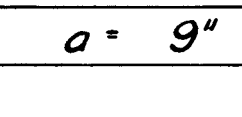
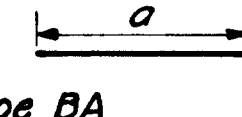
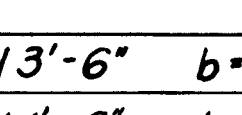
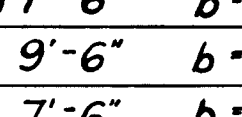
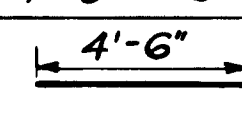
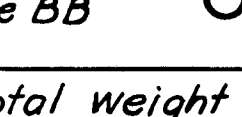


REINFORCEMENT SCHEDULE

MARK	SIZE	LENGTH	BENDING DIAGRAM	NO. REQ'D	WT. EACH	TOTAL WEIGHT	LOCATION OF BAR
Outlet Gate Structure							
0519-6	$\frac{5}{8}$ " ϕ	19'-6"	Straight	79	20.3	1604	Roof slab and walls.
0512-0	$\frac{5}{8}$ " ϕ	12'-0"	do	104	12.5	1300	Vertical in walls.
059-6	$\frac{5}{8}$ " ϕ	9'-6"	do	18	9.9	178	Roof slab and walls.
059-0	$\frac{5}{8}$ " ϕ	9'-0"	do	176	9.4	1654	Vertical in walls.
058-0	$\frac{5}{8}$ " ϕ	8'-0"	do	10	8.3	83	Roof slab and center pier.
054-6	$\frac{5}{8}$ " ϕ	4'-6"	do	2	4.7	9	Roof slab.
051-6	$\frac{5}{8}$ " ϕ	1'-6"	do	2	1.6	3	do
054-6AA	$\frac{5}{8}$ " ϕ	4'-6"		64	4.7	301	Vertical, East & West walls.
0511-0BA	$\frac{5}{8}$ " ϕ	11'-0"		4	11.5	46	Transverse roof slabs.
0510-0BA	$\frac{5}{8}$ " ϕ	10'-0"		6	10.4	62	do
057-0BA	$\frac{5}{8}$ " ϕ	7'-0"		2	7.3	15	Longitudinal, top of slab.
056-3BA	$\frac{5}{8}$ " ϕ	6'-3"		2	6.5	13	do
054-6BA	$\frac{5}{8}$ " ϕ	4'-6"		72	4.7	338	Horizontal in corners.
054-0BA	$\frac{5}{8}$ " ϕ	4'-0"		2	4.2	8	Longitudinal, top of slab.
053-9BA	$\frac{5}{8}$ " ϕ	3'-9"		2	3.9	8	do
0525-0CA	$\frac{5}{8}$ " ϕ	25'-0"		5	26.1	131	Transverse roof slab.
0515-0CA	$\frac{5}{8}$ " ϕ	15'-0"		16	15.6	250	Longitudinal, top of slab.
059-6CA	$\frac{5}{8}$ " ϕ	9'-6"		5	9.9	50	Center pier.
056-9LB	1" ϕ	6'-9"		18	18.0	324	Ladder rungs.
0811-0	1" ϕ	11'-0"	Hook	2	29.4	59	Roof slab

MARK	SIZE	LENGTH	BENDING DIAGRAM	NO. REQ'D	WT. EACH	TOTAL WEIGHT	LOCATION OF BAR
Outlet Apron							
089-9	1" ϕ	9'-9"	Straight	46	26.0	1196	Vertical in wall.
0522-0	$\frac{5}{8}$ " ϕ	22'-0"	do	1	23.0	23	Transverse in top slab.
0521-3	$\frac{5}{8}$ " ϕ	21'-3"	do	2	22.2	44	do
0520-6	$\frac{5}{8}$ " ϕ	20'-6"	do	2	21.4	43	do
0520-0	$\frac{5}{8}$ " ϕ	20'-0"	do	2	20.9	42	do
0519-3	$\frac{5}{8}$ " ϕ	19'-3"	do	2	20.1	40	do
0518-9	$\frac{5}{8}$ " ϕ	18'-9"	do	2	19.6	39	do
0511-6	$\frac{5}{8}$ " ϕ	11'-6"	do	20	12.0	240	Longitudinal in walls.
0511-0	$\frac{5}{8}$ " ϕ	11'-0"	do	20	11.5	230	do
059-9	$\frac{5}{8}$ " ϕ	9'-9"	do	24	10.2	245	Vertical in wall.
0822-0BA	1" ϕ	22'-0"		2	58.7	117	Transverse, bottom slab.
0818-0BA	1" ϕ	18'-0"		18	48.1	866	do
0817-3BA	1" ϕ	17'-3"		24	46.1	1106	do
0517-0BA	$\frac{5}{8}$ " ϕ	17'-0"		21	17.7	372	Longitudinal, top of slab.
0516-6BA	$\frac{5}{8}$ " ϕ	16'-6"		2	17.2	34	do
0522-6ED	$\frac{5}{8}$ " ϕ	22'-6"		11	23.5	259	Top and bottom.
0517-3LB	$\frac{5}{8}$ " ϕ	17'-3"		21	18.0	378	Longitudinal, bottom.
0516-9LB	$\frac{5}{8}$ " ϕ	16'-9"		2	17.5	35	do

MARK	SIZE	LENGTH	BENDING DIAGRAM	NO. REQ'D	WT. EACH	TOTAL WEIGHT	LOCATION OF BAR
Wing Walls at Outlet							
079-6	$\frac{5}{8}$ " ϕ	9'-6"	Straight	16	19.4	310	Transverse in footing.
068-0	$\frac{5}{8}$ " ϕ	8'-0"	do	18	12.0	216	do
066-6	$\frac{5}{8}$ " ϕ	6'-6"	do	18	9.8	176	do
0521-6	$\frac{5}{8}$ " ϕ	21'-6"	do	4	22.4	90	Longitudinal
0520-6	$\frac{5}{8}$ " ϕ	20'-6"	do	4	21.4	86	do
0518-9	$\frac{5}{8}$ " ϕ	18'-9"	do	56	19.6	1098	do
0515-0	$\frac{5}{8}$ " ϕ	15'-0"	do	16	15.6	250	do
0514-9	$\frac{5}{8}$ " ϕ	14'-9"	do	4	15.4	62	do
0513-9	$\frac{5}{8}$ " ϕ	13'-9"	do	4	14.3	57	do
0512-0	$\frac{5}{8}$ " ϕ	12'-0"	do	8	12.5	100	Vertical in stem
0510-0	$\frac{5}{8}$ " ϕ	10'-0"	do	10	10.4	104	do
058-0	$\frac{5}{8}$ " ϕ	8'-0"	do	56	8.3	465	Vert. in stem and long.
057-6	$\frac{5}{8}$ " ϕ	7'-6"	do	4	7.8	31	Longitudinal
056-6	$\frac{5}{8}$ " ϕ	6'-6"	do	34	6.8	231	do
056-0	$\frac{5}{8}$ " ϕ	6'-0"	do	22	6.3	139	Vertical in stem
055-0	$\frac{5}{8}$ " ϕ	5'-0"	do	32	5.2	166	Transverse in footing
053-9	$\frac{5}{8}$ " ϕ	3'-9"		24	3.9	94	Dowels
0818-0BA	1" ϕ	18'-0"		12	48.1	577	Footing to stem.
0815-6BA	1" ϕ	15'-6"		18	41.4	745	do
0613-0BA	$\frac{5}{8}$ " ϕ	13'-0"		16	19.5	312	do
0510-6BA	$\frac{5}{8}$ " ϕ	10'-6"		28	11.0	308	do
0812-9BB	1" ϕ	12'-9"		12	34.0	408	do
Total weight on this sheet						17,446	pounds

REINFORCEMENT SCHEDULE SUMMARY

SCHEDULE	WEIGHT
Total weight on Steel Reinforcement No. 1 (Item No. 11)	19,046 pounds
Total weight on Steel Reinforcement No. 2 (Item No. 11)	11,659 pounds
Total weight on Steel Reinforcement No. 3 (Item No. 11)	79,721 pounds
Total weight on Steel Reinforcement No. 4 (Item No. 11)	53,001 pounds
Total weight on Steel Reinforcement No. 5 (Item No. 11)	74,749 pounds
Total weight on Steel Reinforcement No. 6 (Item No. 11)	12,850 pounds
Total weight on Steel Reinforcement No. 7 (Item No. 11)	277,362 pounds
Total weight on Steel Reinforcement No. 8 (Item No. 11)	17,446 pounds
Grand total (Item No. 11)	545,834 pounds

NOTES

All reinforcing steel to be paid for under Item No. 11.
All bends to be made around a pin with a diameter of 8 times the diameter of the bar unless otherwise noted.

12-31-45	As built revisions				
DATE	REVISION	REVBY	CK. BY	AP. BY	

CONNECTICUT RIVER FLOOD CONTROL			
NORTH MEADOWS PUMPING STATION			
FISCAL YEAR 1939 SECTION			
STEEL REINFORCEMENT NO. 8			
HARTFORD, CONN.			
CONNECTICUT RIVER,		CONNECTICUT	
IN 62 SHEETS		SHEET NO. 43	
NOT TO SCALE			
U.S. ENGINEER OFFICE PROVIDENCE, R.I. FEB. 1939			
SUBMITTED: <i>Wm. B. Nichols</i>		APPROVED: <i>J. D. Burns</i>	
SENIOR ENGINEER		PRINCIPAL ENGINEER	
HEAD, DESIGN SECTION		CHIEF, F.C. ENGINEERING DIV.	
DESIGNED: <i>E. M. Vassar</i>		DRAWN BY: <i>N. I.</i>	
ASSOC. ENGINEER		CHECKED BY: <i>G. H. B.</i>	
		FILE NO. CT-4-1291	